





# Lots 154 & 155 Cnr Alexander Drive and Landsdale Road, East Landsdale Proposed Commercial Development Transport Impact Assessment

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### 1. Introduction and Background

Transcore prepared a TIA in 2018 for the proposed Amendment to the East Landsdale Structure Plan for the proposed Local Centre on Lots 154 and 155 located at the north-west corner of the Alexander Drive and Landsdale Road intersection (subject site) in East Landsdale. **Appendix A** illustrates the site plan access arrangement for the proposed Local Centre at that time.

The subject site entails Lots 154 and 155 which are located at the north-west corner of Alexander Drive and Landsdale Road intersection in East Landsdale, as shown in **Figure 1**.

**Figure 2** shows the location of the subject site within the context of the Metropolitan Region Scheme (MRS). As evident Alexander Drive is classified as Other Regional Road (Blue Road) in the MRS.

The proposed Development Application plan (as shown in **Appendix B**) is in line with the proposed access arrangement for the proposed Local Centre except the connections to Ravanello Terrace and Melanzana Chase no longer allow vehicle access to the development in line with local development plan.

The focus of this TIA is the proposed commercial development on Lots 154 and 155 including a service station, a supermarket, fast food outlets and a number of retail shops. As part of the assessments the trip generation and distribution of the proposed development were established and SIDRA intersection analysis were undertaken for the western development crossovers on Landsdale Road and the intersection of Landsdale Road/ Alexander Drive.

The development site plan was reviewed to ensure satisfactory access, egress and circulation for all types of vehicles and in particular fuel tankers. The stacking capacity of the proposed fast-food outlets, liquor store, car wash and the service station were also reviewed.



Figure 1: Location of the subject site

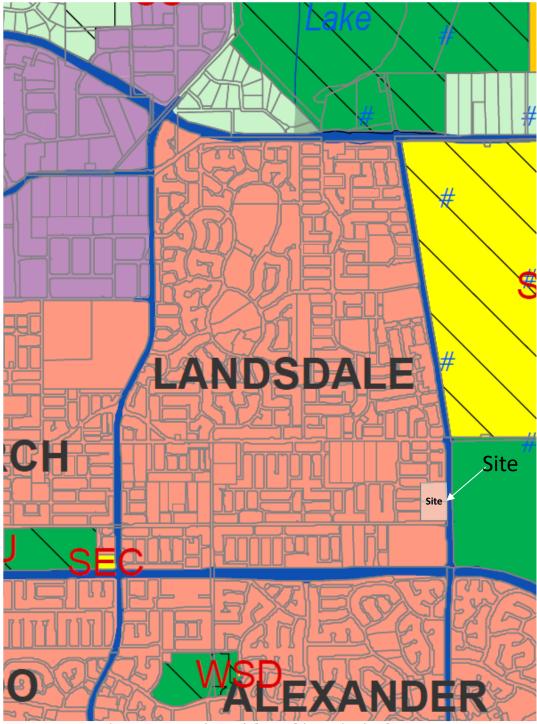


Figure 2: Location of the Subject Site in the MRS

### 2. Existing Situation

### Existing Land Use

The subject site is currently vacant land with one residential dwelling within the southern portion. The subject site entails one formal and one informal crossover on Landsdale Road about 100m and 80m away from the Alexander Drive intersection respectively. Adjacent land uses to the north and west are mainly residential. Hepburn Park is located to the south and Alexander Drive is forming the eastern boundary of the subject site.

### Existing Road Network

The existing road network and its classification in the Main Roads WA functional road hierarchy is illustrated in **Figure 3**.

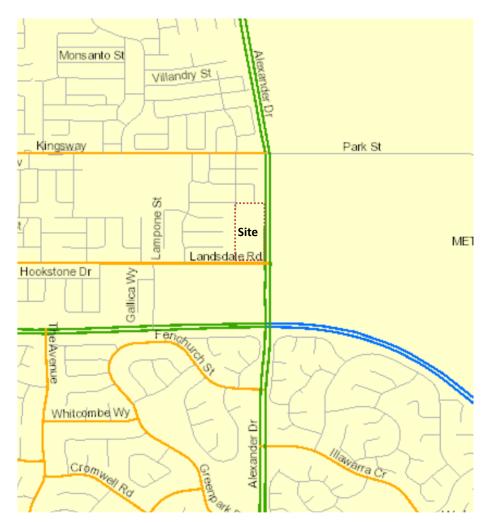


Figure 3: Existing road hierarchy

Landsdale Road in the vicinity of the subject site is constructed with 7.2m pavement and 1m shoulders on both sides. Landsdale Road has a posted speed limit of 60km/h in this vicinity. According to the information obtained from the City of Wanneroo, Landsdale Road east of Pomodora Avenue carried about 1,736vpd in December 2017. The traffic counts undertaken by Transcore for the turn movements of the intersection of Landsdale Road/ Alexander Drive (17 October 2020) indicated that Landsdale Road immediately west of Alexander Drive carried about 106vph, 155vph and 94vph during the Weekday AM, Weekday PM and Saturday mid-day peak hours respectively (refer **Figure 4**).

Landsdale Road intersects with Alexander Drive at a channelised T-intersection with turn pockets on Alexander Drive. Based on advice from the Department of Planning, this intersection is likely to be upgraded to traffic signal control or roundabout sometime in the future as a result of traffic growth on Alexander Drive.

**Alexander Drive** is constructed to dual divided carriageway standard with a wide (about 12m) median and posted speed limit of 70kmh in the vicinity of the subject site. Alexander Drive is classified as a "District Distributor A" road under the Main Road WA functional road hierarchy. This road is also classified as "Other Regional Road" (Blue Road) in the Metropolitan Region Scheme.

### **Existing Traffic Volumes**

The traffic counts undertaken by Transcore for the turn movements of the intersection of Landsdale Road/ Alexander Drive (17 October 2020) is illustrated in **Figure 4.** 

Existing average weekday traffic (AWT) volumes on Alexander Drive (obtained from Main Roads WA) are illustrated in **Figure 5**. The SCATS data for the signalised intersection of Hepburn Avenue and Alexander Drive was also reviewed to establish the historical traffic growth on Alexander Drive. As evident in **Figure 5** the existing traffic counts on Alexander Drive in this vicinity has been reduced by about 13% in 2020 since 2017.

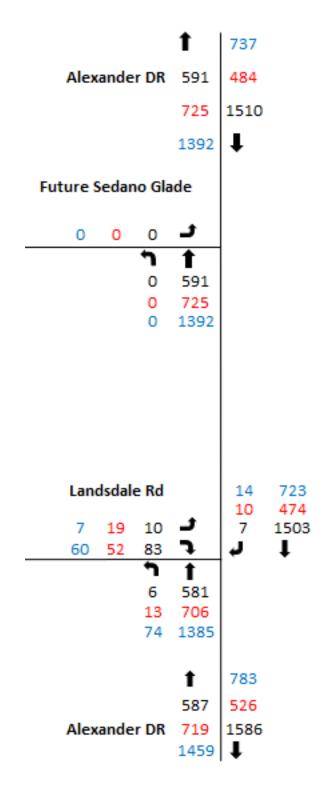


Figure 4: Existing traffic counts AM, PM, Saturday mid-day peak hours

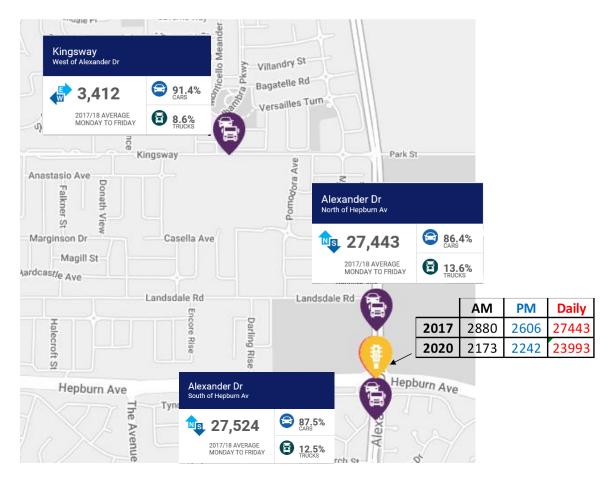


Figure 5: Existing traffic counts

### Heavy Vehicle Routes

Restricted Access Vehicle (RAV) Network routes are designated for access by large heavy vehicle combinations, which is managed by Main Roads WA. Alexander Drive in the vicinity of the subject site forms part of RAV Tandem Drive Network 4 as shown in **Figure 6**. The RAV Tandem Drive Network 4 classification permits a variety of prime mover and trailer combinations, such as B-doubles up to a maximum length of 27.5m.

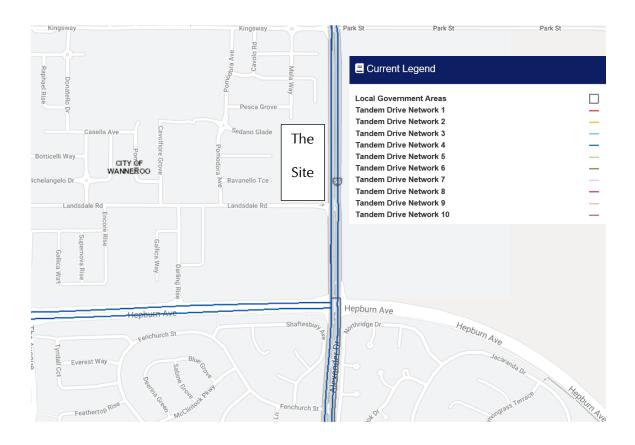


Figure 6: Restricted access vehicle network

### Public Transport

Currently, there are no bus routes servicing the subject site. The closest existing bus route is Bus Route No. 450 along Kingsway Road as shown in **Figure 7** which traverses about 500m to the north of the site.

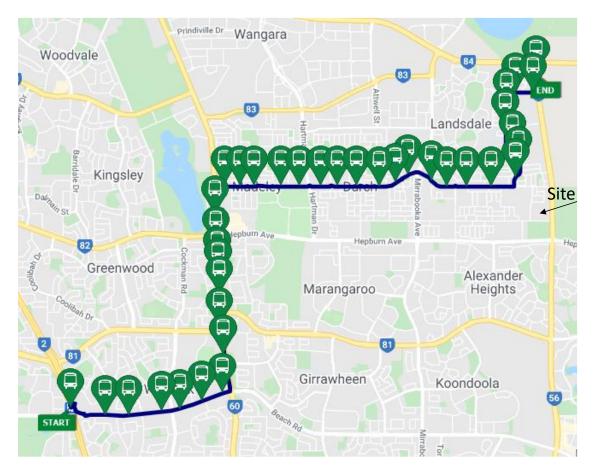


Figure 7: Existing bus route

### Pedestrian and Cyclist Facilities

The Department of Transport's Perth Bike Map series shows good riding environment on Landsdale Road fronting the subject site as shown in **Figure 8**.



### Crash Data

Information available on Main Roads WA website provides crash statistics for Landsdale Road/ Alexander Drive intersection during the five-year period ending in December 2019. The crash records indicate 4 crashes at this intersection with no casualty. More detail on the crash records are provided in **Table 1**.

Intersection	Total Crashes	Casualty			
Landsdale Road/	4	0			
Daylight	Rear End	Rt Angle	Wet	Pedestrian	
3	2	2	4	0	0

Table 1. Crash history for Landsdale Road/ Alexander Drive intersection
---

### **3. Development Proposal**

### **Proposed Site Use**

The proposed Development Concept Plan is shown in **Appendix B.** The proposed development includes:

- A Service station with about 290 m<sup>2</sup> convenience store and eight fuelling positions;
- A Supermarket with total GFA of about 1,846m<sup>2</sup>;
- A Carwash with 1 auto bay, 1 super wash and 4 manual bays;
- Fast food outlets with drive through facilities with total GFA of about 898m<sup>2</sup>,
- Retail shops with total GFA of about 1,302m<sup>2</sup>;
- A Liquor store with total GFA of about 438 m<sup>2</sup>, with about 100 m<sup>2</sup> drive through;
- A Medical centre with total GFA of about 440 m<sup>2</sup>;
- Takeaway outlets with total GFA of about 300 m<sup>2</sup>;
- A Restaurant with total GFA of about 200 m<sup>2</sup>;
- A Pharmacy with total GFA of about 216 m<sup>2</sup>; and,
- Office with total GFA of about 447 m<sup>2</sup>.

The proposed development is in line with the approved local development plan. The proposed development will be connected to Alexander Drive via Landsdale Road and the continuation of Sedano Glade with 3 crossovers on Sedano Glade and two crossovers on Landsdale Road. The access crossovers on Sedano Glade will operate as full movement and would need to be constructed as simple T-intersections. The eastern most crossover onto Sedano Glade would operate as an exit only crossover.

The eastern access crossover on Landsdale Road is proposed to be left in/ left out due to its proximity to the intersection of Landsdale Road/ Alexander Drive. The western access crossover on Landsdale Road would operate as full movement and would need to be constructed as a simple T-intersection. According to the approved local development plan, Mela Way from the north will connect to Sedano Glade and will provide a direct connection to the proposed residential dwellings to the north of the proposed development.

Pedestrian access to the site will be facilitated from the existing footpaths on the abutting road. Footpath connectivity from the existing network to the proposed development is recommended to facilitate the pedestrian movements.

### **Proposed Access for all Modes**

Figure 9 shows the proposed access crossovers on Landsdale Road and Sedano Glade.

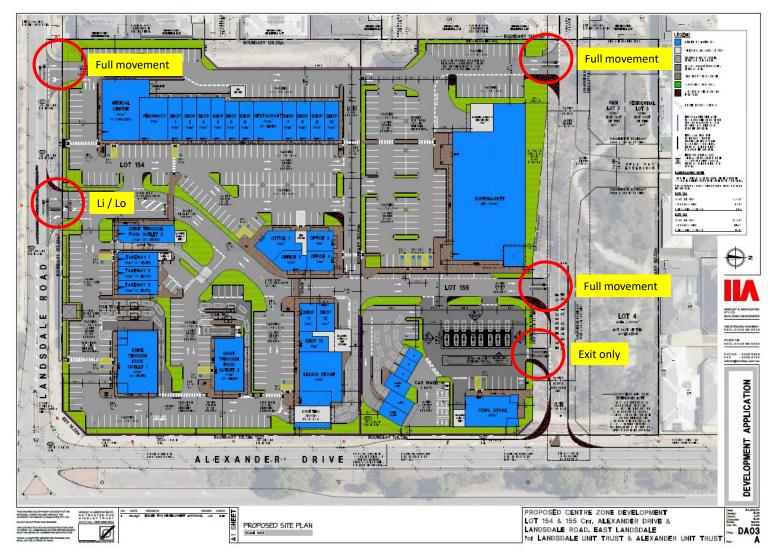


Figure 9: Proposed development access/egress system

### **4.** Changes to the surrounding Road Network

The intersection of Landsdale Road/ Alexander Drive is currently operating as priority-controlled T-intersection which is likely to be upgraded to traffic signals or roundabout in the future. However, at present no timeframe for this upgrade is available.

The proposed intersection of Sedano Glade/ Alexander Drive would operate as a left in/ left out intersection. A kerbed median on Landsdale Road would be required to enforce the left in/ left out treatment at the eastern most crossover. The details of the proposed median (length and width) will be investigated during the detailed design stage of the development.

The turn lane assessment undertaken indicates that a left turn slip lane (approximately 85m including taper) at the proposed left in/ left out intersection of continuation of Sedano Glade/Alexander Drive would be required to satisfy Austroads requirements.

It should be noted that the proposed left turn slip lane and the continuation of Sedano Glade/Alexander Drive (once constructed) would be used by the proposed development and the existing and proposed residential developments in this vicinity.

The extension of the Sedano Glade to Alexander Drive would trigger the requirement for the left turn slip lane regardless of the proposed development. Therefore, the provision of the proposed left turn slip lane would improve safety and traffic operations of the existing and future residents in this vicinity as well as the proposed development.

### **5. Integration with Surrounding Area**

The proposed development is within the approved East Landsdale Structure Plan area and the land uses for the proposed development are predominantly retail/ commercial.

### 6. Traffic Assessment

### Assessment Years and Time Periods

The assessment years that have been adopted for this analysis are 2021 and 2031 in accordance with the requirements of the WAPC Guidelines.

The proposed land uses within the proposed development are predominantly retail/ commercial and, therefore, it is expected that the peak combination of development traffic and road network traffic would occur during the typical weekday afternoon and Saturday mid-day peak period. However, this TIA includes the AM peak hour analysis as well.

### **Development Generation and Distribution**

#### Estimated Existing Traffic Generation

The site is presently mainly vacant and does not generate significant traffic.

#### Proposed Development Traffic Generation

The traffic volumes that will be generated by the proposed development have been estimated using trip generation rates derived from the RTA NSW – Guide to Traffic Generating Developments, TRMS NSW – Guide to Traffic Generating Developments Updated Traffic Surveys 04a (2013) and the Institute of Transport Engineers Trip Generation Manual (10th Edition).

It should be noted that since retail types of land uses typically generate minimal trips during weekday AM peak hour, adjustment factors have been applied for such land uses in an attempt to realistically represent the actual traffic activity during this period. Hence, minimal traffic activity is anticipated for the retail type of uses during AM peak hour.

Due to the land use mix within the proposed development incidences of multipurpose trips<sup>1</sup> (i.e. cross-trade) are anticipated. Accordingly, the applied cross-trade adjustment is calculated to result in a moderate overall reduction in trip generation of approximately 30% but only during the weekday PM and Saturday mid-day peak period and for the overall daily trips.

Accordingly, it is estimated that the proposed development would generate a total of approximately 9,000 total trips per weekday (both inbound and outbound) with

<sup>&</sup>lt;sup>1</sup> Multi-purpose trips are incidences where more than one shop/outlet are visited within the development (also referred to as "cross-trade")

approximately 645, 700 and 1065 trips (both inbound and outbound) during weekday morning, weekday afternoon and Saturday mid-day peak hour periods, respectively as shown in **Table 2**.

**Table 3** summarises the passing trade and primary trips component of the total development generated trips. The net addition of traffic when accounting for passing traffic is approximately +379 trips (AM peak hour), +350 trips (PM peak hour) and +600 trips (Saturday-day peak hour) on the surrounding road network.

Two traffic distributions have been modelled for the weekday AM, PM peak and Saturday mid-day peak hours:

- **4** Passing trade traffic as detailed in **Figure 10**.
- **4** Non-passing trade traffic as detailed in **Figure 11**.

The total proposed development traffic is detailed in **Figure 12**. The development traffic distribution modelled in this report has been evaluated by considering the catchment area of the proposed development, existing traffic patterns and the identified key traffic routes.

1 1	0	Della Dete	Weekd-AM	Sat-PM	Weekd-PM	Cross Trada	Daile Taina	Weekd-AM	Sat-PM	Weekd-PM	A	M	Sat-	PM	P	м
Land use	Quantity	Daily Rate	Peak	Peak	Peak	Cross Trade	Daily Trips	trips	trips	trips	IN	OUT	IN	OUT	IN	OUT
Retail (shops)	1302	0.46	0.00	0.06	0.05	0.30	419	6	54	42	3	3	27	27	21	21
Service Station	8	205.36	12.47	19.46	13.99	0.30	1150	100	109	78	50	50	54	55	39	39
Supermarket	1846	1.15	0.04	0.08	0.10	0.30	1485	76	106	129	38	38	53	53	64	65
Fast food outlet with drive through	898	5.07	0.27	0.55	0.35	0.30	3187	246	348	221	123	123	174	174	111	110
Takeway outlets	300	0.97	0.03	0.52	0.31	0.30	204	9	110	64	5	4	55	55	32	32
Liquore Store	438	1.09	0.00	0.18	0.18	0.30	335	0	57	54	0	0	28	29	27	27
Restaurant	200	1.21	0.01	0.19	0.11	0.30	169	2	26	15	1	1	13	13	7	8
Medical Centre	440	3.73	0.27	0.52	0.04	0.30	1148	119	161	11	59	60	81	80	6	5
Pharmacy	216	0.97	0.03	0.11	0.09	0.30	147	7	17	14	3	4	9	8	7	7
Office	447	0.10	0.02	0.024	0.02	0.30	31	9	8	6	4	5	4	4	3	3
Carwash	5.00	200.00	20.00	20.00	20.00	0.30	700	70	70	70	35	35	35	35	35	35
		TOTAL T	RAFFIC				8975	644	1065	704	321	323	533	532	352	352

Table 2: AM Weekday, Lunch time Saturday and PM Weekday peak hour trip generation for the proposed land uses

Table 3: Passing trade and primary trips components of the trip generation

	Passing Trade Component									
Passing Trade		А	M	S	at-PM	PM				
	Daily Trips	IN	OUT	IN	OUT	IN	OUT			
34%	143	1	1	9	9	7	7			
60%	690	30	30	32	33	23	24			
36%	535	14	14	19	19	23	23			
50%	1593	62	62	87	87	56	55			
50%	102	3	2	28	28	16	16			
89%	298	0	0	25	25	24	24			
43%	73	0	0	6	6	3	3			
0%	0	0	0	0	0	0	0			
53%	78	2	2	5	4	4	4			
0%	0	0	0	0	0	0	0			
60%	420	21	21	21	21	21	21			
	3932	133	132	232	232	177	177			

Primary Trips Component

	А	М	Sat-PM PM			М
Daily Trips	IN	OUT	IN OUT		IN	OUT
276	2	2	18	18	14	14
460	20	20	22	22	16	15
950	24	24	34	34	41	42
1594	61	61	87	87	55	55
102	2	2	27	27	16	16
37	0	0	3	4	3	3
96	1	1	7	7	4	5
1148	59	60	81	80	6	5
69	1	2	4	4	3	3
31	4	5	4	4	3	3
280	14	14	14	14	14	14
5043	188	191	301	300	175	175

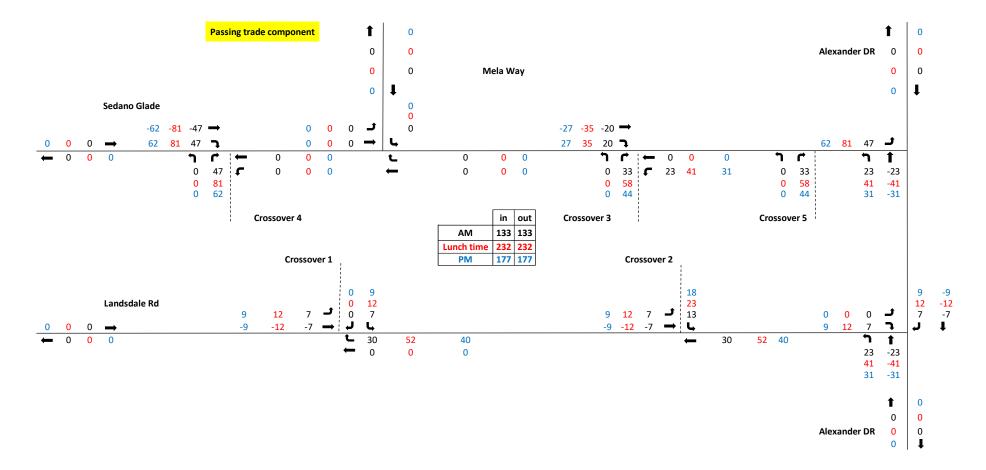


Figure 10: Passing trips, AM weekday, mid-day Saturday and PM weekday peak hour traffic for the proposed development

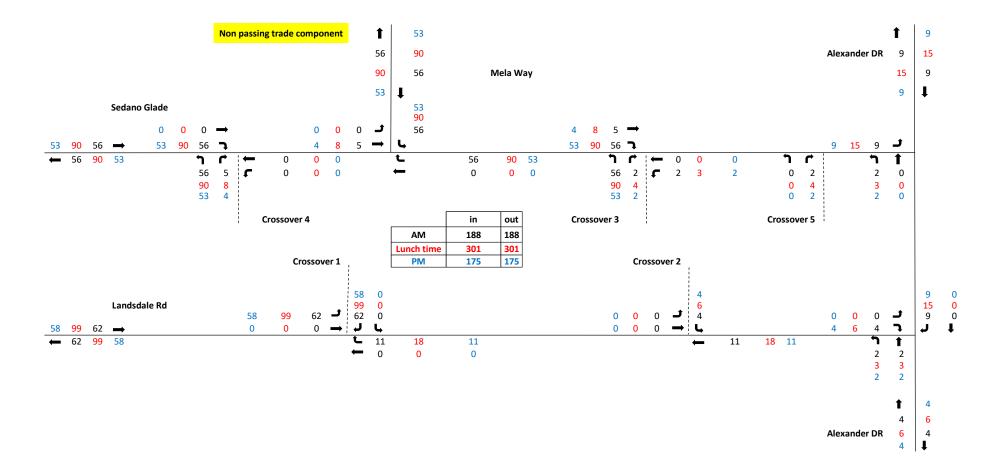


Figure 11: Additional (non-passing trade) component - AM weekday, mid-day Saturday and PM weekday peak hours

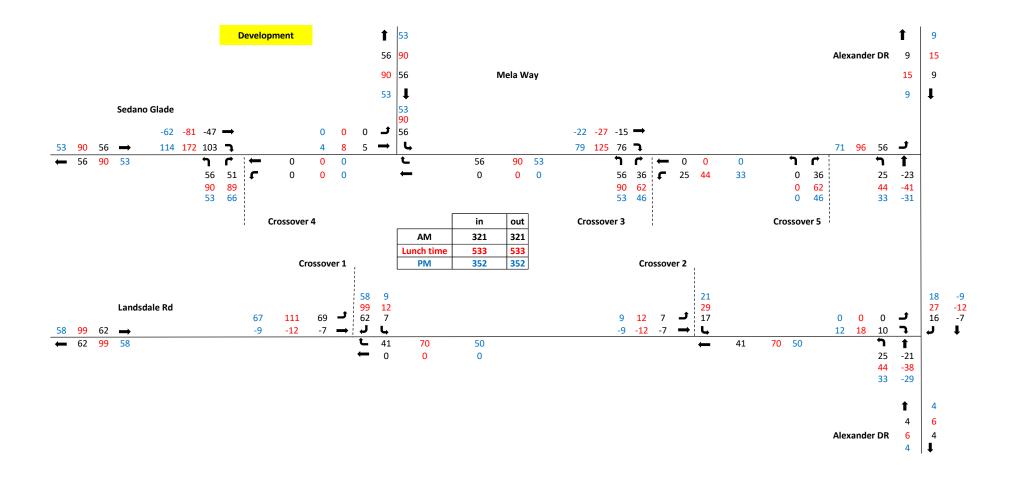


Figure 12: Total peak hour traffic generated by the proposed redevelopment – AM weekday, mid-day Saturday and PM weekday peak hours

### **Traffic Flow Forecasts**

The existing traffic flows are presented in **Figure 13**. The existing traffic volumes were derived from traffic survey conducted on 17<sup>th</sup> October 2020 by Transcore and traffic counts available from Main Roads WA.

The combined base and development traffic volumes for the post-redevelopment scenario are presented in **Figure 14**.

To approximate the year 2031 traffic on Landsdale Road a traffic growth of 20% was applied to the tuning movements of the intersection of Landsdale Road/ Alexander Drive but no growth has been applied to the Alexander Drive traffic because review of the historical traffic counts showed traffic reduction along this section of Alexander Drive (refer **Figure 5**).

The total ten-year post-redevelopment traffic volumes are presented in Figure 15.

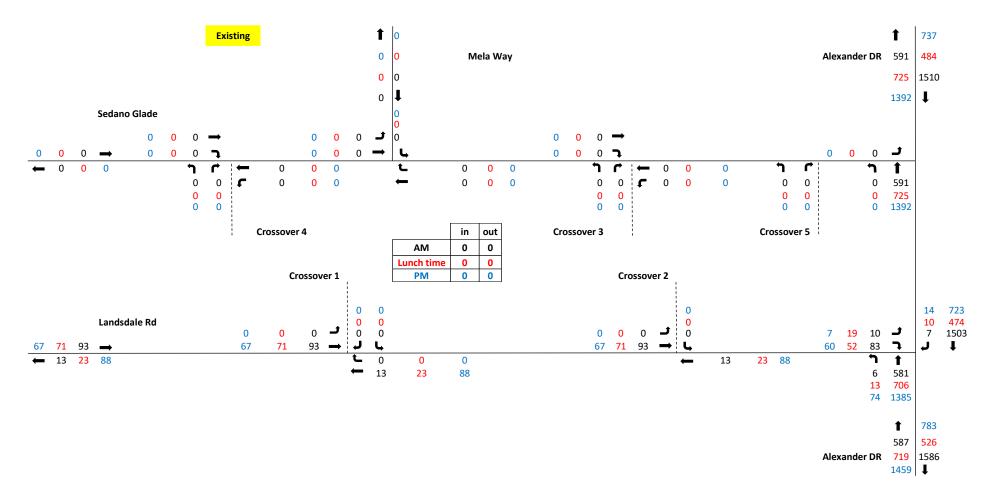


Figure 13: Existing traffic flows near the subject site – AM weekday, Lunch time Saturday and PM weekday peak hours

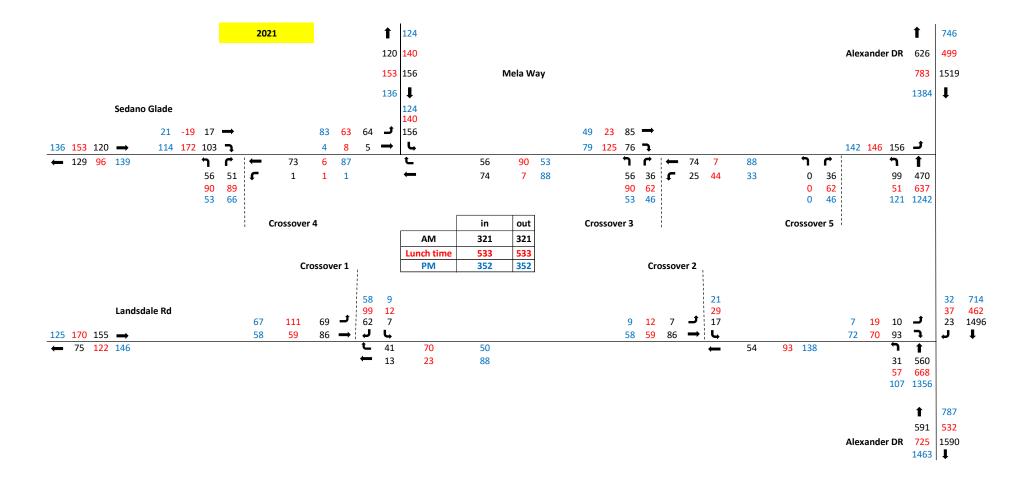


Figure 14: Post-development traffic flows near the subject site – AM weekday, mid-day Saturday and PM weekday peak hours

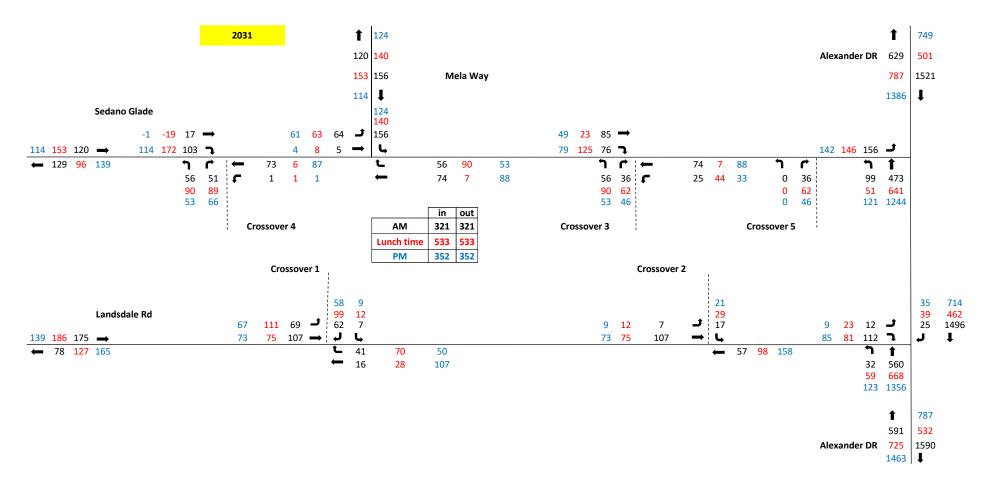


Figure 15: Estimated 10-year total post-development traffic flows near the subject site – AM weekday, mid-day Saturday and PM weekday peak hours

### Intersection Analysis

Capacity analysis was undertaken for the existing priority-controlled intersection of Alexander Drive/ Landsdale Road and the proposed development full movement crossover on Landsdale Road (western crossover) for weekday AM and PM and Saturday mid-day peak hours. The eastern crossover is Left in/ Left out crossover and therefore SIDRA analysis was not undertaken for this crossover.

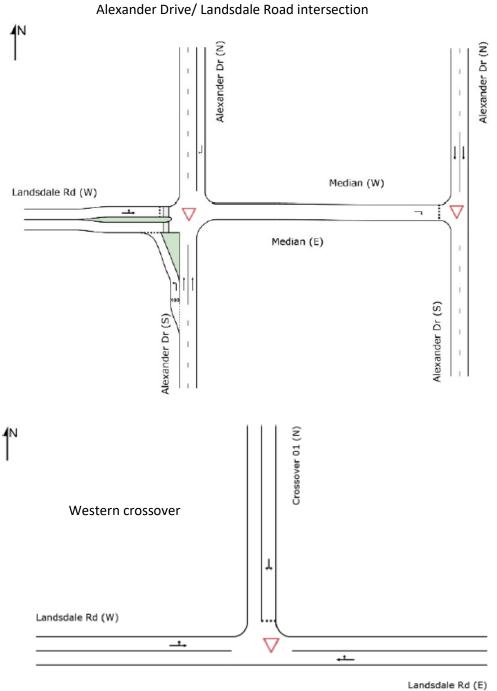
Capacity analysis was undertaken using the SIDRA computer software package. SIDRA is an intersection modelling tool commonly used by traffic engineers for all types of intersections. SIDRA outputs are presented in the form of Degree of Saturation, Level of Service, Average Delay and 95% Queue. These characteristics are defined as follows:

- Degree of Saturation (DoS): is the ratio of the arrival traffic flow to the capacity of the approach during the same period. The Degree of Saturation ranges from close to zero for varied traffic flow up to one for saturated flow or capacity.
- Level of Service (LoS): is the qualitative measure describing operational conditions within a traffic stream and the perception by motorists and/or passengers. In general, there are 6 levels of service, designated from A to F, with Level of Service A representing the best operating condition (i.e. free flow) and Level of Service F the worst (i.e. forced or breakdown flow).
- Average Delay: is the average of all travel time delays for vehicles through the intersection.
- **95% Queue**: is the queue length below which 95% of all observed queue lengths fall.

The results of the SIDRA analysis are summarised in **Appendix C**. The SIDRA intersection models were coded with reference to the Main Roads WA Operation Modelling Guidelines Version No. 1.1. All relevant parameters such as heavy vehicle groups, PCU factors etc. were coded as per the Main Roads WA Guidelines.

The modelled geometry of the intersection and crossover in SIDRA are shown in **Figure 16**. As evident the existing wide median on Alexander Drive allows for two stage movement for the right turns out of Landsdale Road.

The full movement crossover on Landsdale Road is modelled as basic T-intersection in SIDRA.



Lanusuale Ku (

Figure 16: Sidra layouts

#### Alexander Drive/ Landsdale Road Intersection

The SIDRA analysis results indicate that the intersection of Alexander Drive/ Landsdale Road presently operates satisfactorily with moderate queues and delays for the right turn movements in and out of Landsdale Road during weekday PM peak hour (refer **Appendix C** for more details).

The addition of the development-generated traffic to the intersection resulted in negligible increases in overall queues and delays. No significant change in LoS for any of the turns is reported during the post-development scenario (refer **Appendix C** for more details).

The SIDRA assessment for the 10-year post development during weekday AM and Saturday mid-day peak hour rendered similar results to post-development scenario with again marginal increases in delays and queues and no significant changes to the Level of Service for any of the turns (refer **Appendix C** for more details). SIDRA results indicated that the right turn movements in and out of Landsdale Road during the PM peak hours would experience more delays. However, the level of queues and delays is not reported as significant.

It should be noted that the upgrades to the intersection (roundabout or traffic lights) as outlined in the Structure Plan would improve the capacity and traffic operations of the intersection.

#### Proposed Crossover

The SIDRA analysis results indicate that the proposed development western crossover on Landsdale Road will operate at an overall LoS A for both post-development and 10-year post-development scenarios.

### Turn lane assessment

In order to investigate the need for a left turn slip lane on Alexander Drive for the proposed left in/ left out intersection, reference was made to the warrants in "Austroads Guide to Road Design Part 4" document and MRWA Supplement to Austroads Guide to Road Design - Part 4.

The assessment undertaken by Transcore using MRWA spreadsheet tool for 2031 projected traffic volumes indicates that a AUL or CHL treatment would be required for the left turn movement at this intersection. Therefore, it is recommended to provide an 85m (including taper) left turn slip lane at the intersection.

### Impact on Surrounding Roads

**Table 4** summarises the 2031 projected traffic volumes on the surrounding roadsusing the projected traffic volumes reported in **Figure 15**.

Roads	AM (vph)	PM (vph)	Sat mid-day (vph)
Landsdale Rd (west of the Alexander Drive)	180	250	200
Landsdale Rd (west of the development)	250	300	310
Sedano Glade (West of Alexander Drive)	255	260	200
Mela Way	275	235	290

 Table 4: 2031 projected peak hour traffic volumes on surrounding roads

Accordingly, Sedano Glade and Mela Way would carry less than 300vph or 3,000vpd in future and the proposed "Access Street" standard of these roads as indicated in the road hierarchy plan for the East Landsdale Structure Plan would be able to accommodate the projected traffic volumes on these roads. Accordingly, the original proposed 18m road reserve for these roads would still be adequate to accommodate the additional traffic generated by the proposed development.

Mela Way is constructed with a 6.0 m wide carriageway within the 18.0m road reserve which is suitable for low densities (<R40) environment and will provide occasional on-street parking without disruption to the through traffic flow.

Sedano Glade is planned to be a bus route and therefore requires a 7.2m carriageway width. The existing sections of this road have already been constructed to a 7.2m carriageway width standard and it is proposed that the same standard carriageway width be constructed towards east.

The Structure Plan proposes a "Higher Order Access Street" classification for Landsdale Road. However, due to its function and the projected traffic volumes it would be more appropriate if this road is classified as "Neighbourhood Connector A" road. The existing 20m road reservation of this road would be able to accommodate the projected traffic volumes and the proposed new classification without the construction of any on-street parking.

The internal site layout including the proposed accessways are designed to accommodate the waste collection trucks or service vehicles in both directions and accommodate the projected traffic volumes within the site.

### Impact on Neighbouring Areas

The traffic generated by the proposed development is not expected to significantly affect surrounding areas and the road network has been designed to accommodate this type and level of development traffic.

### Traffic Noise and Vibration

It generally requires a doubling of traffic volumes on a road to produce a perceptible 3dB (A) increase in road noise. The proposed development will not increase traffic volumes on surrounding roads anywhere near this level.

## 7. Parking

The development proposes to provide a total of 302 and 131 for car parking spaces for Lot 154 and 155 respectively.

It is considered that the proposed parking provision is sufficient to accommodate the needs of the proposed development.

### 8. Provision for Heavy Vehicles

The fuel delivery trucks are expected to enter and exit the proposed service station via the proposed left in/ left out intersection on Alexander Drive. This intersection has been designed to accommodate the 19.0m fuel tankers. The turn path analysis confirms satisfactory traffic movements of the fuel tanker.

The relevant turn path analysis was also undertaken to ensure satisfactory operation of the service vehicles entering and exiting the loading dock areas for each individual loading dock within the development. The outcomes of the turn path analysis are shown in **Appendix D**.

# 9. Stacking Capacity

The stacking capacity of the proposed fast-food outlets and the proposed car wash was reviewed against the RTA Guidelines requirements. A queue length analysis model was developed for the proposed service station to investigate the queuing capacity of the proposed service station.

#### **RTA requirements**

Section 5.8.1 of RTA Traffic Generating Developments document deals with the parking requirements for the drive-in and take-away food outlets. With respect to the drive through facilities this section states that:

An exclusive area for queuing of cars for a drive through is required (queue length of 5 to 12 cars measured from pick up point). There should also be a minimum of four car spaces for cars queued from the ordering point.

The proposed fast-food outlets 1 and 2 provide a drive through facility with two Customer Order Booth (COB). The fast-food outlet 1 provides for 14 car stacking capacity with 6 car spaces available from the ordering points (3 on each side). The fast-food outlet 2 provides for 12 car stacking capacity with 4 car spaces available from the ordering points (2 on each side).

The fast-food outlet 3 is a small size fast-food with seven stacking car capacity and minimum of four car spaces for cars queued from the ordering point in line with RTA requirements.

Accordingly, the proposed drive through facilities for both fast-food outlets meet and exceed the RTA drive through requirements.

The drive through of the proposed liquor store also provides 19 car stacking capacity which is expected to be sufficient.

The RTA guidelines provide design requirements for single unit car wash and suggest that each unit should be able to accommodate at least five cars. The proposed development has three manual and one automated tunnel and one super wash which allows for a quick reduction in queuing due to availability of services. The proposed carwash can accommodate at least 21 cars (five cars stacking capacity for each manual bay and six cars stacking capacity for the auto bay). So, the stacking capacity is considered sufficient for the proposed carwash.

#### Queue Length Analysis Model

The stacking capacity of the proposed service station and detailed queue analysis at the filling points have been assessed to investigate the impacts of higher-thanaverage site patronage during peak operational periods. This analysis was undertaken to confirm the capacity of the service station to operate satisfactory under amplified traffic activity conditions (e.g. "cheap fuel" day).

Based on the peak hour trip generation documented in this TIA, it is estimated that the proposed service station would attract up to 56 vehicles during the regular weekday PM peak hour (critical peak hour). In order to ensure a robust assessment, it is assumed that the trade on "cheap fuel" day would be 50% higher than the typical peak weekday PM hour. Accordingly, it is conservatively assumed that the site would attract about 84 cars per hour on this occasion.

The experience indicates that, under normal circumstances, the rate of service per fill point (time taken for a vehicle to arrive, park at a fill point, get fuel, pay for fuel and leave the fill point and service station site) is usually between 2-3 minutes. In some circumstances refuelling time may extend to about five minutes when window washing or other similar activities are practiced. However, during the "cheap fuel" day periods and due to high turnover of vehicles and "pressure" from the patrons waiting behind the parked vehicle to access the bowser, the refuelling activity is always shortened and typically in order of up to 3min maximum. In this case, and in order to allow for a conservative scenario, the service time is assumed to be 4 minutes. Accordingly, a service rate of 240sec (15 vehicles per hour) was assumed for weekday PM peak "cheap fuel" peak hour.

It is assumed that all bowsers will be in operation during the peak periods, giving an order taking service rate and capacity of 120 vehicles per hour. It is also assumed that cars would enter the service channel with the shortest queue, therefore over the peak hour the transactions at each service channel would be evenly split.

A queue length analysis was undertaken to assess the provision of storage for vehicles within the service channels. For this purpose, an M/M/1 queuing model was adopted for each bowser. The M/M/1 is a single-server queue model that can be used to approximate simple systems.

The queuing model adopts the following assumptions:

- **4** Vehicles arrive unevenly following Poisson's probability distribution;
- **4** Service time is exponentially distributed;
- 4 There is one server per queue, i.e. there are 8 queues, one for each bowser;
- The capacity of the queue in which arriving users wait before being served is infinite (for the purposes of identifying queue space requirements);
- The population of users (i.e. the pool of users) available to join the system is infinite; and,
- **4** The queue is serviced on a first come, first served basis.

The results of the queuing analysis are detailed in Figure 17.

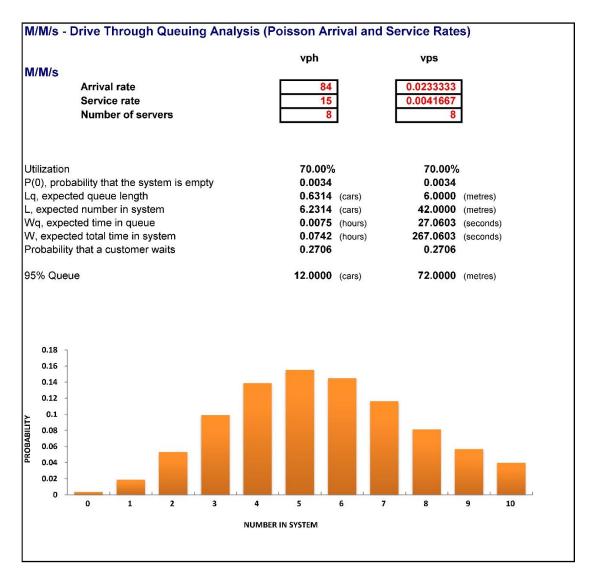


Figure 17. Peak "cheap fuel" hour queuing analysis

In summary, critical "cheap fuel" hour queuing analysis of the service station established the following for the worst-case scenario:

- ↓ The system utilisation is at 70% during the "cheap fuel" hour;
- **4** The expected number in the system (refuelling) is 7 vehicles;
- 4 The expected time in the queue is 267 seconds; and,
- The 95th percentile queue within the whole system is 12 cars (8 cars refuelling and 4 cars waiting).

The queue length usually adopted for robust analysis is the 95th percentile queue. Assuming equal queue distribution it is estimated that in the worst-case scenario there will be about one vehicle waiting behind each refuelling vehicle at four bowsers. The service station forecourt can accommodate this level of queuing.

# 10. Conclusions

The subject of this TIA is the proposed commercial development on Lots 154 and 155 located at the north-west corner of the Alexander Drive and Landsdale Road intersection in East Landsdale.

The proposed development is in line with the approved local development plan. The proposed development will be connected to Alexander Drive via Landsdale Road and the continuation of Sedano Glade with 3 crossovers on Sedano Glade and two crossovers on Landsdale Road.

The eastern access crossover on Landsdale Road is proposed to be left in/ left out due to its proximity to the intersection of Landsdale Road/ Alexander Drive. The western access crossover on Landsdale Road would operate as full movement and would need to be constructed as a basic T-intersection.

The net addition of traffic when accounting for passing traffic is approximately +379 trips (AM peak hour), +350 trips (PM peak hour) and +600 trips (Saturday-day peak hour) on the surrounding road network.

The SIDRA analysis results indicate that the intersection of Alexander Drive/ Landsdale Road presently operates satisfactorily with moderate queues and delays. The addition of the development-generated traffic to the intersection would result in minor increases in overall queues. It should be noted that the future planned upgrades to the intersection (roundabout or traffic lights) would improve the capacity and traffic operations of the intersection.

The SIDRA analysis results indicate that the proposed development western crossover on Landsdale Road (as a basic priority-controlled T-intersection) will operate at an overall LoS A for both post-development and 10-year post-development scenarios.

The turn lane assessment using MRWA spreadsheet tool for 2031 projected traffic volumes indicates that a AUL or CHL treatment would be required for the left turn movement at the proposed left in/ left out intersection on Alexander Drive. Therefore, it is recommended to provide an 85m left turn slip lane (including taper) at the intersection to satisfy Main Roads WA and Austroads requirements.

The standard and road hierarchy of Mela Way, Sedano Glade and Landsdale Road as indicated in the proposed road hierarchy plan for the East Landsdale Structure Plan would be able to accommodate the projected traffic volumes on these roads after full development of the proposed local centre.

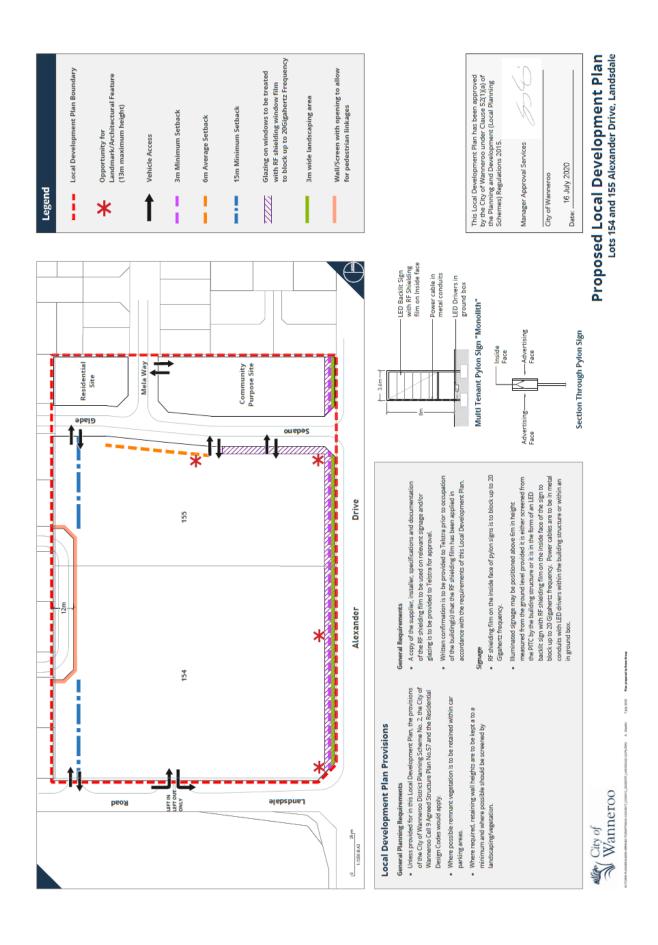
Turn path analysis undertaken for fuel tanker and service vehicles confirm satisfactory access, egress and circulation.

The stacking capacity assessment for the proposed fast-food outlets and the car wash confirms satisfactory traffic operations. The queue assessments undertaken for the service station confirms that the site layout would be able accommodate internal patronage with no blockage of the service station crossovers.

It is considered that the proposed parking provision is sufficient to accommodate the needs of the proposed development.

# Appendix A

# ENDORSED LOCAL DEVELOPMENT PLAN



# Appendix **B**

# **DEVELOPMENT APPLICATION PLAN**



# **Appendix C**

SIDRA ANALYSIS

V Site: [Alexander Dr & Landsdale Rd - Stage 1 - Existing - ♠♥ Network: N101 [Existing - AM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance ·	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% Bacl Queue		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	H∨ %	v/c	sec		Vehicles Dis veh	stance m		Rate	Cycles \$	Speed km/h
South	n: Alexa	ander Dr (S	3)											
4	L2	6	2.0	6	2.0	0.004	6.6	LOSA	0.0	0.1	0.04	0.57	0.04	46.2
5	T1	612	14.3	612	14.3	0.182	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	bach	618	14.2	618	14.2	0.182	0.1	LOSA	0.0	0.1	0.00	0.01	0.00	69.6
North	: Alexa	nder Dr (N	1)											
12	R2	7	2.0	7	2.0	0.008	8.2	LOSA	0.0	0.2	0.55	0.64	0.55	30.2
Appro	bach	7	2.0	7	2.0	0.008	8.2	NA	0.0	0.2	0.55	0.64	0.55	30.2
West	Lands	dale Rd (\	N)											
1	L2	11	2.0	11	2.0	0.171	5.1	LOSA	0.6	4.7	0.57	0.76	0.57	29.3
2	T1	87	2.0	87	2.0	0.171	8.0	LOSA	0.6	4.7	0.57	0.76	0.57	19.9
Appro	bach	98	2.0	98	2.0	0.171	7.7	LOSA	0.6	4.7	0.57	0.76	0.57	21.6
All Ve	hicles	723	12.4	723	12.4	0.182	1.2	NA	0.6	4.7	0.08	0.12	0.08	64.9

## **MOVEMENT SUMMARY**

 ✓ Site:
 [Alexander Dr & Landsdale Rd - Stage 2 - Existing ♥♥ Network: N101 [Existing - AM]

 AM]

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn		Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	peed km/h
North	: Alexa	nder Dr (N	I)											
11	T1	1582	15.1	1582	15.1	0.473	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	bach	1582	15.1	1582	15.1	0.473	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.8
West	: Media	an (W)												
3	R2	87	2.0	87	2.0	0.478	27.1	LOS D	1.6	11.9	0.93	1.04	1.21	4.1
Appro	bach	87	2.0	87	2.0	0.478	27.1	LOS D	1.6	11.9	0.93	1.04	1.21	4.1
All Ve	hicles	1669	14.4	1669	14.4	0.478	1.4	NA	1.6	11.9	0.05	0.05	0.06	65.2

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Queu		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	H∨ %	v/c	sec		Vehicles D veh	istance m		Rate	Cycles	Speed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	78	2.0	78	2.0	0.049	6.6	LOSA	0.2	1.5	0.06	0.56	0.06	46.0
5	T1	1458	14.3	1458	14.3	0.434	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	bach	1536	13.7	1536	13.7	0.434	0.4	LOSA	0.2	1.5	0.00	0.03	0.00	68.2
North	: Alexa	nder Dr (N	1)											
12	R2	15	2.0	15	2.0	0.063	21.3	LOS C	0.2	1.6	0.87	0.95	0.87	18.1
Appro	bach	15	2.0	15	2.0	0.063	21.3	NA	0.2	1.6	0.87	0.95	0.87	18.1
West	Lands	dale Rd (\	N)											
1	L2	7	2.0	7	2.0	0.632	28.1	LOS D	2.3	17.5	0.96	1.09	1.42	8.6
2	T1	63	2.0	63	2.0	0.632	59.6	LOS F	2.3	17.5	0.96	1.09	1.42	3.8
Appro	bach	71	2.0	71	2.0	0.632	56.3	LOS F	2.3	17.5	0.96	1.09	1.42	4.4
All Ve	hicles	1621	13.1	1621	13.1	0.632	3.0	NA	2.3	17.5	0.05	0.08	0.07	59.9

## **MOVEMENT SUMMARY**

Mov	ement	Perform	ance -	Vehi	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay			Back of eue	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles	Speed km/h
North	n: Alexa	ander Dr (N	1)											
11	T1	761	15.1	761	15.1	0.228	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	oach	761	15.1	761	15.1	0.228	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9
West	: Media	an (W)												
3	R2	63	2.0	63	2.0	0.094	5.9	LOSA	0.3	2.2	0.55	0.80	0.55	13.6
Appro	oach	63	2.0	63	2.0	0.094	5.9	LOSA	0.3	2.2	0.55	0.80	0.55	13.6
All Ve	ehicles	824	14.1	824	14.1	0.228	0.5	NA	0.3	2.2	0.04	0.06	0.04	68.1

♥ Site: [Alexander Dr & Landsdale Rd - Stage 1 - Existing -Sat Lunch time]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance ·	- Vehio	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quei		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h	%	Total veh/h	HV %	v/c	sec		Vehicles E veh	)istance m		Rate	Cycles S	Speed km/h
South	n: Alexa	ander Dr (S	3)											
4	L2	14	2.0	14	2.0	0.009	6.6	LOSA	0.0	0.3	0.05	0.57	0.05	46.1
5	T1	743	14.3	743	14.3	0.221	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	bach	757	14.1	757	14.1	0.221	0.1	LOSA	0.0	0.3	0.00	0.01	0.00	69.3
North	: Alexa	inder Dr (N	I)											
12	R2	11	2.0	11	2.0	0.014	9.2	LOSA	0.1	0.4	0.60	0.70	0.60	28.8
Appro	bach	11	2.0	11	2.0	0.014	9.2	NA	0.1	0.4	0.60	0.70	0.60	28.8
West:	: Lands	dale Rd (\	N)											
1	L2	20	2.0	20	2.0	0.144	5.4	LOSA	0.5	3.8	0.59	0.76	0.59	28.0
2	T1	55	2.0	55	2.0	0.144	9.8	LOSA	0.5	3.8	0.59	0.76	0.59	18.5
Appro	bach	75	2.0	75	2.0	0.144	8.6	LOSA	0.5	3.8	0.59	0.76	0.59	22.1
All Ve	hicles	842	12.9	842	12.9	0.221	1.0	NA	0.5	3.8	0.06	0.09	0.06	65.6

#### MOVEMENT SUMMARY

♥ Site: [Alexander Dr & Landsdale Rd - Stage 2 - Existing -Sat Lunch time] ₱₱ Network: N101 [Existing -Sat Lunch time]

Move	ement	Perform	ance	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay		95% Bac Queu		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Di veh	stance m		Rate	Cycles S	peed km/h
North	: Alexa	inder Dr (N	I)											
11	T1	499	15.1	499	15.1	0.149	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
Appro	bach	499	15.1	499	15.1	0.149	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0
West:	Media	an (W)												
3	R2	55	2.0	55	2.0	0.060	4.3	LOSA	0.2	1.5	0.43	0.67	0.43	16.7
Appro	ach	55	2.0	55	2.0	0.060	4.3	LOS A	0.2	1.5	0.43	0.67	0.43	16.7
All Ve	hicles	554	13.8	554	13.8	0.149	0.4	NA	0.2	1.5	0.04	0.07	0.04	68.1

V Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2021 - AM] 👘 Network: N101 [2021 - AM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance ·	· Vehio	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Queue		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	tance m		Rate	Cycles S	opeed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	37	2.0	37	2.0	0.023	6.7	LOS A	0.1	0.7	0.08	0.56	0.08	45.9
5	T1	584	14.3	584	14.3	0.174	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	bach	621	13.6	621	13.6	0.174	0.4	LOSA	0.1	0.7	0.00	0.03	0.00	68.0
North	: Alexa	nder Dr (N	1)											
12	R2	24	2.0	24	2.0	0.025	8.1	LOSA	0.1	0.8	0.54	0.68	0.54	30.6
Appro	bach	24	2.0	24	2.0	0.025	8.1	NA	0.1	0.8	0.54	0.68	0.54	30.6
West	: Lands	sdale Rd (\	N)											
1	L2	11	2.0	11	2.0	0.192	5.3	LOSA	0.7	5.3	0.57	0.77	0.57	29.3
2	T1	99	2.0	99	2.0	0.192	8.2	LOSA	0.7	5.3	0.57	0.77	0.57	20.5
Appro	bach	109	2.0	109	2.0	0.192	7.9	LOSA	0.7	5.3	0.57	0.77	0.57	21.8
All Ve	hicles	755	11.5	755	11.5	0.192	1.7	NA	0.7	5.3	0.10	0.16	0.10	62.3

#### **MOVEMENT SUMMARY**

Mov	ement	t Perform	ance	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles I veh	Distance m		Rate	Cycles S	Speed km/h
North	: Alexa	ander Dr (N	I)											
11	T1	1574	15.1	1574	15.1	0.471	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	oach	1574	15.1	1574	15.1	0.471	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.8
West	: Media	an (W)												
3	R2	99	2.0	99	2.0	0.533	28.2	LOS D	1.8	13.7	0.93	1.05	1.28	4.3
Appro	bach	99	2.0	99	2.0	0.533	28.2	LOS D	1.8	13.7	0.93	1.05	1.28	4.3
All Ve	ehicles	1673	14.3	1673	14.3	0.533	1.7	NA	1.8	13.7	0.06	0.06	0.08	64.4

▽ Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2021 - PM] 🛛 🖶 Network: N101 [2021 - PM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bao Queu		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	H∨ %	v/c	sec		Vehicles D veh	istance m		Rate	Cycles S	Speed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	112	2.0	112	2.0	0.071	6.7	LOSA	0.3	2.2	0.10	0.56	0.10	45.8
5	T1	1428	14.3	1428	14.3	0.425	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	bach	1540	13.4	1540	13.4	0.425	0.5	LOSA	0.3	2.2	0.01	0.04	0.01	67.4
North	: Alexa	nder Dr (N	1)											
12	R2	33	2.0	33	2.0	0.130	20.7	LOS C	0.4	3.3	0.87	0.95	0.87	18.8
Appro	ach	33	2.0	33	2.0	0.130	20.7	NA	0.4	3.3	0.87	0.95	0.87	18.8
West:	Lands	sdale Rd (\	N)											
1	L2	7	2.0	7	2.0	0.749	39.8	LOS E	3.1	23.3	0.97	1.17	1.71	7.4
2	T1	76	2.0	76	2.0	0.749	71.6	LOS F	3.1	23.3	0.97	1.17	1.71	3.4
Appro	ach	83	2.0	83	2.0	0.749	68.8	LOS F	3.1	23.3	0.97	1.17	1.71	3.8
All Ve	hicles	1656	12.6	1656	12.6	0.749	4.3	NA	3.1	23.3	0.07	0.11	0.11	56.1

#### MOVEMENT SUMMARY

V Site: [Alexander Dr & Landsdale Rd - Stage 2 - 2021 - PM] 👘 Network: N101 [2021 - PM]

Mov	ement	Perform	ance ·	- Vehio	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay		95% B Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
North	n: Alexa	nder Dr (N	1)											
11	T1	752	15.1	752	15.1	0.225	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	oach	752	15.1	752	15.1	0.225	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9
West	: Media	an (W)												
3	R2	76	2.0	76	2.0	0.111	6.0	LOS A	0.4	2.7	0.55	0.80	0.55	14.4
Appro	oach	76	2.0	76	2.0	0.111	6.0	LOSA	0.4	2.7	0.55	0.80	0.55	14.4
All Ve	ehicles	827	13.9	827	13.9	0.225	0.6	NA	0.4	2.7	0.05	0.07	0.05	67.7

 ✓ Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2021 - Sat
 ♦♦ Network: N101 [2021 - Sat

 Lunch time]
 Lunch time]

Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	t Perform	ance ·	- Vehio	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Queue	of	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	H∨ %	v/c	sec		Vehicles Dis veh	tance m		Rate	Cycles S	Speed km/h
South	n: Alexa	ander Dr (S	5)											
4	L2	59	2.0	59	2.0	0.038	6.7	LOS A	0.2	1.2	0.11	0.55	0.11	45.7
5	T1	704	14.3	704	14.3	0.210	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9
Appro	oach	763	13.3	763	13.3	0.210	0.5	LOS A	0.2	1.2	0.01	0.04	0.01	67.4
North	i: Alexa	ander Dr (N	I)											
12	R2	38	2.0	38	2.0	0.046	9.0	LOS A	0.2	1.4	0.59	0.75	0.59	29.3
Appro	oach	38	2.0	38	2.0	0.046	9.0	NA	0.2	1.4	0.59	0.75	0.59	29.3
West	: Lands	sdale Rd (V	N)											
1	L2	20	2.0	20	2.0	0.185	5.6	LOS A	0.7	5.0	0.61	0.78	0.61	27.7
2	T1	73	2.0	73	2.0	0.185	10.2	LOS B	0.7	5.0	0.61	0.78	0.61	18.6
Appro	oach	93	2.0	93	2.0	0.185	9.2	LOSA	0.7	5.0	0.61	0.78	0.61	21.5
All Ve	ehicles	894	11.7	894	11.7	0.210	1.8	NA	0.7	5.0	0.10	0.15	0.10	62.1

### MOVEMENT SUMMARY

Mov	ement	t Perform	ance ·	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Queu		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles D veh	istance m		Rate	Cycles S	Speed km/h
North	: Alexa	ander Dr (N	I)											
11	T1	487	15.1	487	15.1	0.146	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
Appro	oach	487	15.1	487	15.1	0.146	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0
West	: Media	an (W)												
3	R2	73	2.0	73	2.0	0.079	4.4	LOS A	0.3	1.9	0.43	0.68	0.43	17.4
Appro	oach	73	2.0	73	2.0	0.079	4.4	LOSA	0.3	1.9	0.43	0.68	0.43	17.4
All Ve	ehicles	560	13.4	560	13.4	0.146	0.6	NA	0.3	1.9	0.06	0.09	0.06	67.4

# ▽ Site: [Landsdale Rd & Crossover 1 - 2021 - AM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformanc	e - Vel	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Landsda	le Rd (E)										
8	T1	14	2.0	0.038	0.5	LOS A	0.2	1.3	0.26	0.43	0.26	46.8
9	R2	47	2.0	0.038	6.0	LOS A	0.2	1.3	0.26	0.43	0.26	28.6
Appro	ach	61	2.0	0.038	4.7	NA	0.2	1.3	0.26	0.43	0.26	32.7
North	: Crosso	ver 01 (N)										
10	L2	8	2.0	0.056	0.3	LOS A	0.2	1.4	0.23	0.18	0.23	26.1
12	R2	55	2.0	0.056	1.0	LOS A	0.2	1.4	0.23	0.18	0.23	32.7
Appro	ach	63	2.0	0.056	0.9	LOS A	0.2	1.4	0.23	0.18	0.23	31.9
West:	Landsda	ale Rd (W)										
1	L2	63	2.0	0.079	5.6	LOS A	0.0	0.0	0.00	0.25	0.00	36.9
2	T1	89	2.0	0.079	0.0	LOS A	0.0	0.0	0.00	0.25	0.00	53.4
Appro	ach	153	2.0	0.079	2.3	NA	0.0	0.0	0.00	0.25	0.00	45.3
All Ve	hicles	277	2.0	0.079	2.5	NA	0.2	1.4	0.11	0.27	0.11	39.0

## MOVEMENT SUMMARY

# ▽ Site: [Landsdale Rd & Crossover 1 - 2021 - PM]

Move	ement F	Performanc	e - Ve	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Landsda	ale Rd (E)										
8	T1	93	2.0	0.081	0.2	LOS A	0.3	2.3	0.18	0.21	0.18	52.4
9	R2	52	2.0	0.081	5.9	LOS A	0.3	2.3	0.18	0.21	0.18	32.0
Appro	ach	144	2.0	0.081	2.3	NA	0.3	2.3	0.18	0.21	0.18	45.1
North	Crosso	over 01 (N)										
10	L2	9	2.0	0.067	0.2	LOS A	0.2	1.7	0.24	0.20	0.24	26.1
12	R2	62	2.0	0.067	1.3	LOS A	0.2	1.7	0.24	0.20	0.24	32.6
Appro	ach	72	2.0	0.067	1.1	LOS A	0.2	1.7	0.24	0.20	0.24	31.9
West:	Landsd	ale Rd (W)										
1	L2	71	2.0	0.069	5.6	LOS A	0.0	0.0	0.00	0.32	0.00	36.0
2	T1	61	2.0	0.069	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	51.8
Appro	ach	132	2.0	0.069	3.0	NA	0.0	0.0	0.00	0.32	0.00	42.2
All Ve	hicles	347	2.0	0.081	2.3	NA	0.3	2.3	0.12	0.25	0.12	40.6

# ▽ Site: [Landsdale Rd & Crossover 1 - 2021 - Sat Lunch time]

Move	ment F	erformanc	e - Vel	hicles								
Mov ID	Turn	Demand f Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate		Average Speed km/h
East: I	Landsda	ale Rd (E)										
8	T1	24	2.0	0.061	0.6	LOS A	0.3	2.2	0.29	0.43	0.29	46.9
9	R2	73	2.0	0.061	6.1	LOS A	0.3	2.2	0.29	0.43	0.29	28.7
Appro	ach	97	2.0	0.061	4.7	NA	0.3	2.2	0.29	0.43	0.29	33.2
North:	Crosso	ver 01 (N)										
10	L2	12	2.0	0.106	0.2	LOS A	0.4	2.8	0.25	0.21	0.25	26.0
12	R2	103	2.0	0.106	1.2	LOS A	0.4	2.8	0.25	0.21	0.25	32.6
Appro	ach	115	2.0	0.106	1.1	LOS A	0.4	2.8	0.25	0.21	0.25	32.0
West:	Landsd	ale Rd (W)										
1	L2	115	2.0	0.094	5.6	LOS A	0.0	0.0	0.00	0.38	0.00	35.3
2	T1	63	2.0	0.094	0.0	LOS A	0.0	0.0	0.00	0.38	0.00	50.4
Appro	ach	178	2.0	0.094	3.6	NA	0.0	0.0	0.00	0.38	0.00	39.7
All Vel	hicles	389	2.0	0.106	3.1	NA	0.4	2.8	0.15	0.34	0.15	35.7

V Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2031 - AM] 🕈 🖶 Network: N101 [2031 - AM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance -	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bao Queu		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Di veh	stance m		Rate	Cycles S	Speed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	39	2.0	39	2.0	0.025	6.7	LOSA	0.1	0.7	0.08	0.56	0.08	45.9
5	T1	584	14.3	584	14.3	0.174	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	bach	623	13.5	623	13.5	0.174	0.4	LOSA	0.1	0.7	0.01	0.03	0.01	67.9
North	: Alexa	nder Dr (N	1)											
12	R2	25	2.0	25	2.0	0.026	8.1	LOSA	0.1	0.8	0.54	0.68	0.54	30.6
Appro	ach	25	2.0	25	2.0	0.026	8.1	NA	0.1	0.8	0.54	0.68	0.54	30.6
West:	Lands	dale Rd (\	N)											
1	L2	13	2.0	13	2.0	0.239	5.5	LOSA	0.9	6.7	0.58	0.79	0.61	28.9
2	T1	118	2.0	118	2.0	0.239	8.5	LOSA	0.9	6.7	0.58	0.79	0.61	20.0
Appro	ach	131	2.0	131	2.0	0.239	8.2	LOSA	0.9	6.7	0.58	0.79	0.61	21.4
All Ve	hicles	779	11.2	779	11.2	0.239	2.0	NA	0.9	6.7	0.12	0.18	0.12	61.3

#### MOVEMENT SUMMARY

▽ Site: [Alexander Dr & Landsdale Rd - Stage 2 - 2031 - AM] ؋ Network: N101 [2031 - AM]

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay			Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
North	: Alexa	ander Dr (N	1)											
11	T1	1574	15.1	1574	15.1	0.471	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	oach	1574	15.1	1574	15.1	0.471	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.8
West	: Media	an (W)												
3	R2	118	2.0	118	2.0	0.635	31.7	LOS D	2.3	17.6	0.95	1.10	1.46	3.9
Appro	oach	118	2.0	118	2.0	0.635	31.7	LOS D	2.3	17.6	0.95	1.10	1.46	3.9
All Ve	ehicles	1692	14.2	1692	14.2	0.635	2.2	NA	2.3	17.6	0.07	0.08	0.10	62.8

V Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2031 - PM] 👘 Network: N101 [2031 - PM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bao Queu		Prop. Queued	Effective Stop	Aver. / No.	lverag e
		Total veh/h		Total veh/h	H∨ %	v/c	sec		Vehicles Di veh	istance m		Rate	Cycles S	peed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	128	2.0	128	2.0	0.082	6.7	LOSA	0.3	2.6	0.11	0.55	0.11	45.7
5	T1	1428	14.3	1428	14.3	0.425	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.8
Appro	bach	1557	13.3	1557	13.3	0.425	0.6	LOSA	0.3	2.6	0.01	0.05	0.01	67.1
North	: Alexa	inder Dr (N	1)											
12	R2	37	2.0	37	2.0	0.147	20.8	LOS C	0.5	3.8	0.88	0.95	0.88	18.7
Appro	bach	37	2.0	37	2.0	0.147	20.8	NA	0.5	3.8	0.88	0.95	0.88	18.7
West	: Lands	sdale Rd (\	N)											
1	L2	9	2.0	9	2.0	0.906	72.9	LOS F	5.0	38.1	0.99	1.37	2.53	5.2
2	T1	89	2.0	89	2.0	0.906	106.2	LOS F	5.0	38.1	0.99	1.37	2.53	2.3
Appro	bach	99	2.0	99	2.0	0.906	103.0	LOS F	5.0	38.1	0.99	1.37	2.53	2.6
All Ve	hicles	1693	12.4	1693	12.4	0.906	7.0	NA	5.0	38.1	0.09	0.14	0.18	50.1

#### **MOVEMENT SUMMARY**

Mov	ement	Perform	ance -	· Vehio	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay		95% Ba Quei		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles E veh	)istance m		Rate	Cycles \$	Speed km/h
North	r: Alexa	ander Dr (N	1)											
11	T1	752	15.1	752	15.1	0.225	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	69.9
Appro	oach	752	15.1	752	15.1	0.225	0.0	NA	0.0	0.0	0.00	0.00	0.00	69.9
West	: Media	an (W)												
3	R2	89	2.0	89	2.0	0.131	6.1	LOS A	0.4	3.2	0.55	0.80	0.55	14.3
Appro	oach	89	2.0	89	2.0	0.131	6.1	LOSA	0.4	3.2	0.55	0.80	0.55	14.3
All Ve	ehicles	841	13.7	841	13.7	0.225	0.7	NA	0.4	3.2	0.06	0.09	0.06	67.2

 ✓ Site: [Alexander Dr & Landsdale Rd - Stage 1 - 2031 - Sat
 ♦♦ Network: N101 [2031 - Sat

 Lunch time]
 Lunch time]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Perform	ance ·	- Vehic	les									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bao Queu		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h	%	Total veh/h	HV %	v/c	sec		Vehicles Di veh	istance m		Rate	Cycles \$	Speed km/h
South	n: Alexa	ander Dr (S	S)											
4	L2	62	2.0	62	2.0	0.040	6.7	LOS A	0.2	1.2	0.11	0.55	0.11	45.7
5	T1	704	14.3	704	14.3	0.210	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	69.9
Appro	bach	766	13.3	766	13.3	0.210	0.6	LOS A	0.2	1.2	0.01	0.04	0.01	67.3
North	: Alexa	nder Dr (N	1)											
12	R2	40	2.0	40	2.0	0.049	9.0	LOS A	0.2	1.5	0.59	0.75	0.59	29.3
Appro	bach	40	2.0	40	2.0	0.049	9.0	NA	0.2	1.5	0.59	0.75	0.59	29.3
West	Lands	sdale Rd (\	N)											
1	L2	24	2.0	24	2.0	0.219	5.7	LOS A	0.8	6.1	0.62	0.79	0.64	27.4
2	T1	85	2.0	85	2.0	0.219	10.5	LOS B	0.8	6.1	0.62	0.79	0.64	18.3
Appro	bach	109	2.0	109	2.0	0.219	9.4	LOS A	0.8	6.1	0.62	0.79	0.64	21.2
All Ve	hicles	916	11.5	916	11.5	0.219	2.0	NA	0.8	6.1	0.11	0.16	0.11	61.3

### MOVEMENT SUMMARY

V Site: [Alexander Dr & Landsdale Rd - Stage 2 - 2031 - Sat ↓ Lunch time] ↓ Lunch time]

Mov	ement	Perform	ance	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quei		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles D veh	istance) m		Rate	Cycles S	Speed km/h
North	i: Alexa	nder Dr (N	1)											
11	T1	487	15.1	487	15.1	0.146	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	70.0
Appro	oach	487	15.1	487	15.1	0.146	0.0	NA	0.0	0.0	0.00	0.00	0.00	70.0
West	: Media	an (W)												
3	R2	85	2.0	85	2.0	0.093	4.4	LOSA	0.3	2.3	0.43	0.68	0.43	17.3
Appro	oach	85	2.0	85	2.0	0.093	4.4	LOS A	0.3	2.3	0.43	0.68	0.43	17.3
All Ve	ehicles	573	13.1	573	13.1	0.146	0.7	NA	0.3	2.3	0.06	0.10	0.06	66.9

# $\nabla$ Site: [Landsdale Rd & Crossover 1 - 2031 - AM]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformanc	e - Ve	hicles								
Mov ID	Turn	Demand f Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Landsda	le Rd (E)										
8	T1	17	2.0	0.040	0.5	LOS A	0.2	1.4	0.28	0.42	0.28	47.1
9	R2	47	2.0	0.040	6.0	LOS A	0.2	1.4	0.28	0.42	0.28	28.8
Appro	ach	64	2.0	0.040	4.6	NA	0.2	1.4	0.28	0.42	0.28	33.6
North	: Crosso	ver 01 (N)										
10	L2	8	2.0	0.058	0.3	LOS A	0.2	1.5	0.26	0.20	0.26	26.0
12	R2	55	2.0	0.058	1.1	LOS A	0.2	1.5	0.26	0.20	0.26	32.6
Appro	ach	63	2.0	0.058	1.0	LOSA	0.2	1.5	0.26	0.20	0.26	31.8
West:	Landsda	ale Rd (W)										
1	L2	63	2.0	0.090	5.6	LOS A	0.0	0.0	0.00	0.22	0.00	37.2
2	T1	111	2.0	0.090	0.0	LOS A	0.0	0.0	0.00	0.22	0.00	54.1
Appro	ach	174	2.0	0.090	2.0	NA	0.0	0.0	0.00	0.22	0.00	46.7
All Ve	hicles	301	2.0	0.090	2.4	NA	0.2	1.5	0.11	0.26	0.11	40.2

## **MOVEMENT SUMMARY**

## ▽ Site: [Landsdale Rd & Crossover 1 - 2031 - PM]

Move	ement F	Performanc	e - Vel	hicles								
Mov ID	Turn	Demand f Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Landsda	ale Rd (E)										
8	T1	113	2.0	0.092	0.2	LOS A	0.3	2.4	0.17	0.18	0.17	53.0
9	R2	52	2.0	0.092	6.0	LOS A	0.3	2.4	0.17	0.18	0.17	32.4
Appro	ach	164	2.0	0.092	2.0	NA	0.3	2.4	0.17	0.18	0.17	46.5
North	: Crosso	ver 01 (N)										
10	L2	9	2.0	0.069	0.2	LOS A	0.2	1.8	0.27	0.22	0.27	25.9
12	R2	62	2.0	0.069	1.4	LOS A	0.2	1.8	0.27	0.22	0.27	32.5
Appro	ach	72	2.0	0.069	1.3	LOS A	0.2	1.8	0.27	0.22	0.27	31.8
West:	Landsd	ale Rd (W)										
1	L2	71	2.0	0.077	5.6	LOS A	0.0	0.0	0.00	0.28	0.00	36.4
2	T1	77	2.0	0.077	0.0	LOS A	0.0	0.0	0.00	0.28	0.00	52.5
Appro	ach	147	2.0	0.077	2.7	NA	0.0	0.0	0.00	0.28	0.00	43.6
All Ve	hicles	383	2.0	0.092	2.1	NA	0.3	2.4	0.12	0.23	0.12	41.8

# ▽ Site: [Landsdale Rd & Crossover 1 - 2031 - Sat Lunch time]

Move	ement F	Performanc	e - Vel	hicles								
Mov ID	Turn	Demand f Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Landsda	ale Rd (E)										
8	T1	29	2.0	0.065	0.6	LOS A	0.3	2.3	0.30	0.41	0.30	47.2
9	R2	73	2.0	0.065	6.1	LOS A	0.3	2.3	0.30	0.41	0.30	28.9
Appro	ach	102	2.0	0.065	4.5	NA	0.3	2.3	0.30	0.41	0.30	34.1
North:	Crosso	ver 01 (N)										
10	L2	12	2.0	0.108	0.3	LOS A	0.4	2.8	0.27	0.23	0.27	25.9
12	R2	103	2.0	0.108	1.3	LOS A	0.4	2.8	0.27	0.23	0.27	32.5
Appro	ach	115	2.0	0.108	1.2	LOS A	0.4	2.8	0.27	0.23	0.27	32.0
West:	Landsd	ale Rd (W)										
1	L2	115	2.0	0.102	5.6	LOS A	0.0	0.0	0.00	0.35	0.00	35.7
2	T1	79	2.0	0.102	0.0	LOS A	0.0	0.0	0.00	0.35	0.00	51.0
Appro	ach	194	2.0	0.102	3.3	NA	0.0	0.0	0.00	0.35	0.00	40.9
All Ve	hicles	411	2.0	0.108	3.0	NA	0.4	2.8	0.15	0.33	0.15	36.5

# **Appendix D**

**TURN PATHS** 

