



 Our Ref:
 BH/SA/L235.21

 Job No:
 21-03-029



Level 2 Kishorn Court 58 Kishorn Road Mount Pleasant WA 6153

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18 November 2021

Landsdale Unit Trust & Alexander Unit Trust PO Box 268 NEDLANDS WA 6009

Attention: Steve Hindley

Dear Steve,

## LOTS 154 & 155 ALEXANDER DRIVE (CNR LANDSDALE RD), EAST LANDSDALE STORMWATER MANAGEMENT PLAN - Rev C

Porter Consulting Engineers (PCE) has been engaged to prepare a stormwater management plan to assist the development application for the above project. The proposed development consists of commercial development and associated parking.

The site is located at Alexander Drive and currently undeveloped land. The site is bounded by residential lots to the north and west. Landsdale Road is situated on the southern boundary. Refer to site location plan shown in **Figure 1** below. The development site is bounded in blue.



Figure 1 – Lots 154 and 155 Alexander Drive, East Landsdale (bound in blue)<sup>1</sup>

<sup>1</sup> MNG Access, viewed on 8 October 2021, <//www.mngaccess.com.au/>

Tusno Pty Ltd ACN 070 097 148 as trustee for the Consulting Engineering Unit Trust trading as Porter Consulting Engineers ABN 78 636 396 385

The commercial development is proposed to consist of a number of retail stores, drive through restaurant outlets, liquor store, a carwash, service station and a supermarket as shown in **Attachment 1**.

**Landform** – The Site is  $32,436m^2$  in area, with an existing home for the former market garden occupying lot 154. The topography of the Site is such that the midpoint of the site is located in a local depression of 42m AHD, with 46m AHD along the northern and southern boundary of the Site.

Geotechnical testing found the permeability very high, in excess of 40m/day. Based on this, a conservative permeability rate on site has been used at 15m/day.

Based on the Perth Groundwater Atlas<sup>2</sup>, the maximum groundwater contour of approximately 40.5m AHD intersects the Site.

The Acid Sulphate Soils (ASS) online risk mapping<sup>3</sup> indicates that the Site is in a "moderate to low risk of ASS occurring within 3m of natural soil surface but high to moderate risk of ASS beyond 3m of natural soil surface". As sewer excavations are not expected to occur beyond 3m depths, for the purpose of this advice, it is assumed ASS management will not be required.

**Drainage** – The City of Wanneroo will require that the development manage and dispose of the 1% AEP (1:100 year) rainfall event within the respective commercial lot boundaries. It is expected the use of soakwells or underground infiltration structures will be utilised to manage the onsite stormwater disposal which would be installed as part of the built-form works. Initial calculations are contained in **Attachment 2**.

For the extension of Sedano Glade and Mela Way, preliminary discussions<sup>4</sup> held with the City of Wanneroo have suggested they would support in principle extension and connection to the existing pit and pipe system in Sedano Glade, which discharges into a fenced sump at the intersection of Sedano Glade and Pomodora Avenue.

A portion of the Sedano Glade runoff near the Alexander Drive intersection is expected to connect to existing drainage in Alexander Drive which discharges into a nearby fenced sump.

For Landsdale Road, there is a local high point approximately midway fronting the development, with existing drainage pits by 357 and 359 Landsdale Road and a drainage pit near Alexander Drive which should avoid the need to install new drainage along Landsdale Road following the kerbing of the development frontage. However, this could be extended if required through the detailed design process.

For the linking extension of Melanzana Chase / Ravanello Terrace, the nearest existing drainage is located at the eastern end of Melanzana Chase. The road connection grades to this drainage pit and therefore connection to this network is readily available for minor catchment increase.

<sup>&</sup>lt;sup>2</sup> Water and Rivers Commission 1997, *Perth Groundwater Atlas*.

<sup>&</sup>lt;sup>3</sup> Department of Water and Environmental Regulations, ASS risk maps, viewed 24 June 2019,

<sup>&</sup>lt;https://www.der.wa.gov.au/your-environment/acid-sulfate-soils/65-ass-risk-maps>

<sup>&</sup>lt;sup>4</sup> Mr A. Baxter 2019, pers comms with S. Highman, 26 June 2019

An indicative layout of the development site is included at **Attachment 3** with suggested number of soakwells for the commercial development. The final drainage design and location would be subject to the detailed design process. It is noted that the road levels and the drainage design would ensure no drainage water crosses boundaries between lots 154 / 155. The residential lots would dispose on site through soakwells and the same would apply to the child care site when this is developed in the future.

## **CONCLUSION**

Based on the information available to us and the strategies set out above, we consider the site capable of meeting the City of Wanneroo requirements for onsite drainage disposal.

If you have any questions regarding this information, please contact the undersigned on 9315 9955.

Yours faithfully,

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RACHEL THOMSON PROJECT ENGINEER

Enc.

ATTACHMENT 1 – Architectural Layout



Proj Job Date Eng Sun Sce File Rev Refe Gro Gro Site	ect Number e ineer nario Name ision erence Do Used und Cond undwater RL	cument	East Lands 21-03-029 24/8/21 RT Soakwell v 1% AEP Co T122.21 B Architect L Landsdale High perm High (RL 4 42-45m									<b>Perfective</b> <b>Consulting Engineers</b> Level 2, 58 Kishorn Road Mount Pleasant WA 6153 PO Box 1036 Canning Bridge WA 6153 Tei: (08) 9315 9955 Fax: (08) 9315 9959 office@portereng.com.au www.portereng.com.au																
Intensities (mm/hr)	1 EY (1 year) 0.2 EY (5 year) 10% AEP (10 Year)	6 minute 30 minute 1 hour 3 hour 6 hour 12 hour 6 minute 1 hour 3 hour 6 hour 12 hour 12 hour 6 minute 30 minute 1 hour 3 hour 6 hour 12 hour 6 hour 12 hour 12 hour 12 hour 10 hou	59.5           25.20           16.30           8.02           5.14           3.29           86.70           36.70           23.50           11.40           7.33           5.58           102.00           43.10           27.60           13.50           8.71           5.58		Co-e <u>Soal</u> Soakw Soakwe Soakwe Soakw	atchmer Are fficient of l well det Liner Dep ective Dep Il base are vell Volum No of Soa <u>Infiltra</u> Soakage (r	at Details         ea (ha) = 3.240         Runoff = 0.90         ails         ther (m) = 1.8         bth (m) = 1.8         oth (m) = 1.8         ea (m <sup>2</sup> ) = 2.54         lee (m <sup>3</sup> ) = 4.58         kkwells = 131         ation         m/day) = 15		Le V Hi Base A	Storage C ength (m) = 20 Width (m) = 20 leight (m) = 0 ume (m <sup>3</sup> ) = 2 Area (m <sup>2</sup> ) = 4	Cells 0.000 0.000 .500 200.00 400.00						Ø1.8m		→	.▲	1.8m	<u>,</u>			20m		0.5m	
		6 minute 30 minute 1 hour	e 159.00 e 66.40 r 42.90	Event Duration	6 min	30 min	1EY (1 Year) 1 hour 3 hour	6 hour	12 hour	6 min 3	30 min	0.2 EY ( 1 hour	(5 Year) 3 hour	6 hour	12 hour	6 min	30 min	10% AEP (1 1 hour 3	0 Year) 3 hour	6 hour	12 hour	6 min	30 min	1 hour	1% AE 3 hour	P (100 Year) 6 hour 12 hou	r 24 hour	72
1	1% AEP	3 hour	22.00	Intensities	59.5	25.2	16.3 8.02	5.14	3.29	86.70	36.7	23.5	11.40	7.33	5.58	102	43.1	27.6	13.50	8.71	5.58	159	66.40	42.90	22.00	14.60 9.4	5.69	

1% AEP 100 Year

 3 hour
 22.00

 6 hour
 14.60

 12 hour
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 24 hour
 5.69

 72 hour
 2.29

ute 66.40	Event 1EY (1 Year)									0.2 EY	' (5 Year)			10% AEP (10 Year)							1% AEP (100 Year)								
our 42.90	Duration	6 min	30 min	1 hour	3 hour	6 hour	12 hour	6 min	30 min	1 hour	3 hour	6 hour	12 hour	6 min	30 min	1 hour	3 hour	6 hour	12 hour	6 min	30 min	1 hour	3 hour	6 hour	12 hour	24 hour	72 hour		
our 22.00	Intensities	59.5	25.2	16.3	8.02	5.14	3.29	86.70	36.7	23.5	11.40	7.33	5.58	102	43.1	27.6	13.50	8.71	5.58	159	66.40	42.90	22.00	14.60	9.4	5.69	2.29		
our 14.60	Q (m <sup>3</sup> /s)	0.4823	0.2043	0.1321	0.0650	0.0417	0.0267	0.7028	0.2975	0.1905	0.0924	0.0594	0.0452	0.8269	0.3494	0.2237	0.1094	0.0706	0.0452	1.2889	0.5383	0.3478	0.1783	0.1184	0.0762	0.0461	0.0186		
our 9.40	Volume	173.64	367.71	475.69	702.15	900.01	1152.16	253.02	535.51	685.81	998.07	1283.48	1954.12	297.67	628.90	805.46	1181.92	1525.12	1954.12	464.01	968.89	1251.96	1926.10	2556.46	3291.88	3985.28	4811.75		
our 5.69	SW Vol.	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04	600.04		
our 2.29	Soak Vol	20.83	104.17	208.35	625.04	1250.08	2500.16	20.83	104.17	208.35	625.04	1250.08	2500.16	20.83	104.17	208.35	625.04	1250.08	2500.16	20.83	104.17	208.35	625.04	1250.08	2500.16	5000.32	15000.95		
	SW Total	620.87	704.21	808.38	1225.08	1850.12	3100.20	620.87	704.21	808.38	1225.08	1850.12	3100.20	620.87	704.21	808.38	1225.08	1850.12	3100.20	620.87	704.21	808.38	1225.08	1850.12	3100.20	5600.35	15600.99		
	Cell Vol	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00	200.00		
	Cell Soak	25.00	125.00	250.00	750.00	1500.00	3000.00	25.00	125.00	250.00	750.00	1500.00	3000.00	25.00	125.00	250.00	750.00	1500.00	3000.00	25.00	125.00	250.00	750.00	1500.00	3000.00	6000.00	18000.00		
	Cell Total	225.00	325.00	450.00	950.00	1700.00	3200.00	225.00	325.00	450.00	950.00	1700.00	3200.00	225.00	325.00	450.00	950.00	1700.00	3200.00	225.00	325.00	450.00	950.00	1700.00	3200.00	6200.00	18200.00		
	Total Vol	845.87	1029.21	1258.38	2175.08	3550.12	6300.20	845.87	1029.21	1258.38	2175.08	3550.12	6300.20	845.87	1029.21	1258.38	2175.08	3550.12	6300.20	845.87	1029.21	1258.38	2175.08	3550.12	6300.20	11800.35	33800.99		
		PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS	PASS		

ATTACHMENT 3 – Concept Drainage Sketch

